Y N N/A		Commercial and/or Light Industrial
	1.	Name of proposed development 24.5.4.7.1
	2.	Name of developer 24.5.4.7.2
	3.	Signature of Civil Engineer, Seal 24.5.4.8; R.S.37:696-LAC19-3:(10.2, 10.3,10.4)
		a. Plat required 24.5.4.6.5; R.S.33:5051
		b. Specifications received 24.5.3.3
	4.	Vicinity map 24.5.4.7.4
	5.	Located by Township, Range and Section 24.5.4.3.7.E
		a. Section, Township, Range, City Limits, and/or Parish Boundaries which abut or cross the proposed subdivision <i>24.5.4.7.8</i>
	6.	Date, scale (1" = 200 ' minimum suggested) and north arrow $24.5.4.7.5$
	7. 8.	Preliminary approval granted and written staff comments submitted 24.5.3.3 Development Improvements Residential
		a. Proposed street names 24.5.4.7.6
		b. Lot and block numbers 24.5.4.7.6
		 c. Alignments of existing streets, rights-of-ways, easements, and servitudes which join or cross the proposed subdivision shown 24.5.4.7.7 1. Right-of-way
		a. 60' minimum 24.7.2.1
		1. Blocks $\leq 1,500$ ' in length rural 24.7.6.3
		 2. Blocks ≤ 600' in length urban 24.7.6.3 2. Roadway
		a. Street jogs with centerline offsets of less than 125' avoided 24.7.6.1.5
		b. Test cylinders (2,750 psi @ 7 days or 4,000 psi @ 28 days) 2 per 500' of pavement 24.7.6.1.9, 24.7.6.1.10
		 Open Ditch - 8" thick, 24' wide PCC pavement or equivalent asphaltic concrete design. 24.7.2.1.a a. Shoulder
		1. 6' wide 8" thick compacted aggregate 24.7.2.1.a
		2. Curb and Gutter (Mandatory inside city)- 8" thick, 27' wide from back-to-back of curb PCC pavement or equivalent asphaltic concrete design. Curb must be roll-over not less than 12" in width and 4" in height and/or barrier type curb not less than 6" in width and 6" in height 24.7.2.1.b
		d. Cul-de-sacs & Turnarounds

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Y N	N/A			Commercial and/or Light Industrial
				1. Cul-de-sacs as per A.A.S.H.T.O. specifications (1984) inside radius \geq 35' $24.7.6.1.6$
				2. Turnarounds 80' wide by 40' each side of centerline 24.7.6.1.6
			e.	Plans use current LADOTD construction standards 24.7.6.1.10
			f.	Street and Traffic signs as per "Louisiana Manual on Uniform Traffic Control Devices" 24.7.6.1.7
			g.	Profiles of all streets 24.5.4.8.3
			h. i.	No more than one lot created at the end of a stubout cross street 24.7.6.3.1 Lots
		0	Б	1. Lot size shall be of such size, including off street parking, so as to accommodate the development 24.7.2.2
		9.		ainage 24.5.50 H
			a. 1	Flood hazard area 24.5.5.9.H
] []		b.	Existing contours at one (1) foot intervals or less shown on final drainage plant 24.5.4.8
			c.	All lots graded to drain to the street or to major drainage arteries as defined by the SDDM 24.7.1.2.6
			d.	Rights-of-way
				1. Definition 22-186
				2. Construction in right-of-way without consent 22-189
				3. Storm drainage pipe shall be located within street right-of-way, special outfall or interconnection right of way may be required 24.7.1.2.6
			e.	outfall or interconnection right-of-way may be required 24.7.1.2.6 Complies with the T.P.C.G. Storm Drainage Design Manual as per 24.7.6.2.6 IV. HYDROLOGY A. Rainfall
				Desgined for 25-year, 24-hour duration as defined by TP40 (Exhibit 3)
				Discharge limited to 10-year, 24-hour pre-development unless downstream improvements are made as to not cause adverse impacts (Exhibit 4)
				B. Hydrologic Data: Preliminary Plan
	_			Vicinity Map
	_			Topographic Map
				Aerial photographs
				Stream flow records
				Historical high water elevations
				FEMA 100 year flood elevation
Page	2			8/1/2008

Y N N/A	Commercial and/or Light Industrial
	Soil types
	Land use
	Slope
	Surface infiltration
	Storage
	C. Coordination: Maximum stage elevation furnished or approved by Terrebonne Parish Engineering DivisionD. Runoff Computation, Hydrograph Development and Modeling:
	1. Rational Method
	Drainage area no greater than 150 acres
	c value taken from Exhibit 5
	DOTD HYDR6020 and HYDR6000 used for storm drain and inlet spacing
	2. Soil Conservation Service (SCS) Method (NRCS) (TR-55)
	Curve Number (CN) taken from Exhibit 5
	Type III, 24-hour rainfall distribution
	Shape factor 256
	3. Unit Hydrograph Method (HEC-1, SWMM, TR-20) E. Flood Routing:
	1. Stream Flow Routing
	2. Reservoir Routing
	F. Land Use
	G. Datum: Elevation referenced to the latest Parish adopted Vertical Datum
	 H. Gage Reading (Historic Data) at major drainage artery V. HYDRAULIC DESIGN A. Storm Design Requirements: 1. Existing site plan:
	Minimum scale 1"=100'
	Drainage features
	1 foot contours
	Utilities
	Roads
	Structures
Page 3	8/1/2008

Y N N/A	Commercial and/or Light Industrial
	Impervious areas
	Flood encroachment areas
2.	Proposed site plan:
	Minimum scale 1"=100'
	Streets
	Utilities
	Drainage features
	Lot lines
	Lot grading
	Discharge canals
3.	Location of major drainage artery Plan/Profile Sheets Drainage
	Horizontal Scale 1"=50' minimum
	Vertical Scale 1"=5' minimum
	Roads
	Horizontal Scale 1"=40' minimum
	Vertical Scale 1"=4' minimum Geometric layout
	Centerline
	Roadway stations
	Finished centerline slopes (0.35% minimum curb and gutter)
	Points of vertical intersection
	Drainpipes
	Size
	Туре
	Invert elevation Structures & Utility lines
	Size
	Туре
	Invert elevation
	Top elevation
	Finished grade at right-of-way
Page 4	8/1/2008

Y N N/A	Commercial and/or Light Industrial
	Hydraulic gradient
	Tailwater elevation
	Ditch flow lines
	Utility lines
	Dimension of all servitudes
	North arrow
	Legend 4. Drainage Map/Hydraulic Computations Drainage Map
	All drainage features
	Right-of-ways and servitudes
	Tributary areas
	Watershed boundaries
	Structure reference numbers
	Discharge points
	North arrow
	Legend
	Hydraulic Computations
	Design criteria
	Rounded to nearest 0.10 foot
	Maximum stages at all nodes
	Tailwater elevation
	Graphic representation of surface and subsurface flow
	Statement of no adverse impact
	Maximum flows (pre vs. post)
	Volume runoff (pre vs. post)
	Hydrographs at discharge points (pre vs. post) (Exhibit 6)
	Runoff factors
	Time of concentration
	Land slope
	Onsite elevation determined by routing flows from downstream tailwater elevation
D 5	0/1/0000

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Y N N/A	Commercial and/or Light Industrial 5. Typical roadway section
	Roadway width
	Roadway thickness
	Shoulder width
	Ditch dimensions
	Ditch side slopes
	Location of all utilities
	Subsurface drainage location
	Right-of-way width
	Transverse road slopes 6. Lot drainage
	Storm drain pipe located within street right-of-way
	Special servitude for interconnection or outfall purposes within subdivision
	All lots inside the Urban Services District and Urban Planning Area graded to drain to the street or to a Major Drainage Artery (Exhibit 1)
	All lots inside Rural Subdivisions graded to drain to the street or to a Major Drainage Artery (Exhibit 1) Outside the Urban Services District and Urban Planning Area the HTRPC can allow a portion to drain to the rear if:
	Drainage is to be perpetually privately maintained, or
	i. Drainage to the rear already exists or is to be dedicated; however, the percentage may not exceed 60% of the total depth of lots up to 225' deep, or that portion greater than 135' on lots greater than 225' deep unless a greater percentage is required to comply with items ii or iii below.
	ii. Where the size limitation of the roadside ditches will be exceeded
	iii. Where the size of the curb and gutter drainage pipe exceeds 36" in diameter
	7. Reference standard plan details of all drainage structures
	8. Existing cross sections at maximum 100' intervals showing:
	Roadway
	Ditch
Page 6	8/1/2008

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Y N N/A	Commercial and/or Light Industrial
	Lot grades
	9. Time of concentration
	a. Rational method
	b. SCS LAG method
	10. South of the South Terrebonne Development Zone
	Minimum roadway elevation +3.5'
	Minimum lot elevation +2.0'
	B. Closed Storm Drainage System
	1. Minimum sizes
	15" minimum diameter
	8" minimum diameter for restrictor pipe2. Minimum Service Life
	Diameter less than 48" 50 year service life
	Diameter greater than or equal to 48" 70 years
	Side drain 30 years
	3. Sized to operate full with a minimum self cleansing velocity4. Slopes
	Maximum slope 10 ft/sec
	Outlet protection for velocity above 10 ft/sec
	5. Manholes or catch basins
	Located at all changed in vertical and horizontal direction
	Maximum Spacing (LaDOTD Hydraulics Manual), but shall not exceed 250'
	Pipe Diameter 3-7 ft/sec 8-12 ft/sec 13-20 ft/sec
	15" 150' 250' 300'
	18" 300' 350' 400'
	24" – 36" 400' 450' 500' 42" and larger 600' 650' 700'
	42 and larger 000 030 700
	6. n value taken from Exhibit 8
	7. Minimum vertical distance of 6" from bottom of pavement to top of drain pipe
	8. All drainpipes under roadway joined in conformance with LaDOTD Type 3 joints
	Catch basins, manholes and grate inlets in conformance with LaDOTE standard plans
Page 7	8/1/2008

Y NN	Commercial and/or Light Industrial 10. Minimum servitude for drain pipe	
	Diameter less than 42" = 15'	
	Diameter 42" and greater = 20' 11. Inlet spacing	
	LaDOTD HYDR6000 used	
	Gutter flow less than 10 cfs	
	Width of flooding less than 8'	
	Spacing less than 250' 12. Pipe size and hydraulic grade line	
	LaDOTD HYDR6020 used	
	Maximum hydraulic clearance at gutter	line of 0.2' above gutter grade
	Design sketches of numbered structures	s& drainage areas provided
	13. Other model with prior approvalC. Open Storm Drainage System1. Minimum sizes	
	15" minimum diameter	
	8" minimum diameter for restrictor pipe 2. Minimum Service Life	e
	Cross drains 50 year service life	
	All Storm drain pipe 70 years	
	Side drain 30 years	
	3. Pipes installed in major drainage arterical allowable headwater of 0.5' or 1.0' belowhichever is less	
	4. Outlet protection for velocity above 10	ft/sec
	5. n value taken from Exhibit 8	
	6. Entrance loss coefficients in conformar Manual	nce with LaDOTD Hydraulics
	7. Minimum vertical distance of 6" from bedrain pipe	pottom of pavement to top of
	8. All drainpipes under roadway joined in Type 3 joints9. Minimum servitude for drain pipe	conformance with LaDOTD
	- 100	
Page 8	••••• 5•••	8/1/2008
		-: -: - -

Y	N N/A	Commercial and/or Light Industrial 10. Roadside ditches
		3:1 side slope
		Maximum depth of 3'-6"
		11. Ditch centerline not less than 12' from edge of roadway
		12. Minimum longitudinal ditch invert slope = 0.001 ft/ft
		13. Minimum road right-of-way with open ditch = 60'
		 LaDOTD HYDR1140 used to determine normal depth of flow in channel
		15. Minimum width of ditch bottom 2'
		16. n for channels taken from Exhibit 8
		17. Water surface profile computed and shown on final drawings18. Culvert sizes
		Future driveway sizes shown on plat
		Culverts sized as though entire subdivision was subsurface
		19. Other model with prior approvalVI. SYSTEM STORAGEA. Detention Facilities:
		1. Greater than 1 acre
		2. Compensatory storage3. Type
		Open basin or pond
		Roof top storage
		Parking lot ponding
		Underground storage
		Uninhabited areas
		Designated as raw land 4. Drainage Plan
		Plan
		Profile
		Cross Section Pipes & Structures
		Size
		Length
		Invert
Pa	age 9	8/1/2008

Y N N/A	Commercial and/or Light Industrial
	Design volume
	Grades
	Bottom Elevation
	Maximum stage elevation
	5. Onsite system designed to handle both on-site runoff and conveyance through the site of off-site runoff
	6. Designed to anticipate, enable and minimize future maintenance needs
	7. Multiple uses encouraged
	8. Visual impacts considered
	9. Adequate access for maintenance personnel
	10. Maximum depth of parking lot detention 8"
	11. Slopes for parking lot detention no less than 1% no more than 3%
	12. Flood surface elevation of parking lot detention at least 1' below the lowest habitable floor elevation of building within 50' of the detention area13. Detention pond slopes
	Interior slope does not exceed 2:1
	Exterior slope does not exceed 3:1
	14. Private benefit = private ownership
	Methods, procedures and guarantees, including appropriate documentation, that the facilities will be perpetually maintained so as to function as designed and not result in nuisances or health hazards 15. Pond dimensions
	If depth is less than 3' deep minimum width $= 6$ '
	If depth is 3' or deeper minimum width = 15'
	16. Landscaped for aesthetic purposes and to stabilize banks
	Seeding and sodding
	No floatable or erodible material (bark mulch) in interior
	17. Failure of owner to maintain will be cause for Parish to perform work and bill owner
	18. Parish maintained pond control structures that do not abut a public right-of-way should be accessible by a 15' minimum right-of-way to allow vehicle access

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Y N	N/A	Commercial and/or Light Industrial	
		19. Control structures designed and constructed to operate automatically as much as possible	
		20. Designed with 1' of freeboard above the elevation of the design flood (except parking lot ponds)21. Pond design	
		Dry - Sloped no flatter than 0.3% toward drainage outlet	
		Wet – "low flow" channel installed with lining at minimum 0.3% slope	
		22. Wet pond bottom elevation 1.5 ft below normal low water elevation if constructed flat	
		23. "Flow through" pond has well defined low flow channel	
		24. Ponds greater than 4' in depth have fence and locked gate 25. Design Volume	
		Shown on plans	
		Storage measured from the on-site 25 year stage elevation to a maximum depth of the pump drawdown elevation	
	-	Wet and dry basins designed so that the portion of their bottom area, which is intended to be dry, shall have standing water no longer than 48 hours for all runoff events equal to or less than the 25-year event	
		26. Hydraulic losses and structural integrity considered in closed systems on private property	
		27. Written restriction on final plat stating that no structure, fill or obstructions shall be located within any drainage easement or delineated flood plain	
		28. All publicly maintained facilities located in a recorded drainage	
		servitude including any necessary for access VII. EROSION AND SEDIMENT CONTROL A. Design:	
		1. Required on all proposed developed sites of one acre or greater	
		2. Incorporated into excavation, construction and post-construction	
		 Provisions for interception of all potential silt-laden runoff made before initial clearing and grading 	
		4. Erosion control and storm water pollution plan provided	
		5. Erosion protection provided for all disturbed areas	
		B. Maintenance agreement provided before building permit is obtained	
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Y	N N/A	Commercial and/or Light Industrial C. Best Management Practices:
		 Existing vegetation preserved where feasible and disturbed portions stabilized as soon as practicable
		2. Structural practices to divert flows from exposed soild, store flows, or otherwise limit runoff and the discharge of pollutants from the site to the extent feasible
		3. Prevention of the discharge of building materials into the Parish storm sewers or waters of the United States
		4. Provide general good housekeeping measures to prevent and contain spills
		 Implementation of proper waste disposal and waste management techniques
		6. Timely maintenance of vegetation, erosion and sediment control
		measures VIII. SERVITUDE REQUIREMENTS AND DEDICATION A. Ditches not adjacent to a roadway
		1. Ditch less than or equal to 4' deep or 18' wide 15' on both sides
		2. Ditch greater than 4' deep and/or 18' wide 15' on one side and 20' on the other
		3. Parallel ditches minimum 20' crown between
		4. Ditch adjacent to roadway not greater than 3.5' and 23' wide5. Minimum servitude for drain pipe
		Diameter less than $42^{\circ} = 15^{\circ}$
		Diameter 42" and greater = 20'
		 B. Letter Of No Objection required for work in parish right-of-way or parish property
		C. Developer's responsibility to record any necessary servitude that are needed to connect a development site with an approved point of discharge
		f. Minimum size and grade of culverts denoted and profiles of all ditches submitted 24.5.4.8.2,3
		- Proposed culverts fit within ditch
		g. Roadside ditch less than 4' deep and less than 18' wide 24.7.6.2.4
		h. Building of bulkheads on Bayou Black (permit) 6-6
		10. Utilities a. Water
		 Fire hydrants – spacing ≤ 300' 24.7.6.1.8
Do	12	9/1/2009

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Y N N/A		Commercial and/or Light Industrial	
		2. Approval letter from Waterworks 24.5.4.6.7; 24.7.5.6	
	b.	Gas	
		1. Gas mains 2" I.D. 3' deep 24.7.5.4.1	
		2. Servitude for gas main provided 24.7.5.4.2	
		3. Approval letter from Gas Utility 24.5.4.6.7	
	c.	Electricity	
		1. Light Standards 22-51	
		a. Standards, "cobra head" or decorative type of appropriate height style and lamping 24.7.5.2	;
		b. Easements 24.7.5.2	
		c. Location, spacing (spacing $300^{\circ} > x > 150^{\circ}$ and one at each intersection within street right of way) $24.7.5.2$	
		3. Approval Letter from Electric Utility 24.5.4.6.7	
	d.	Sewerage	
		1. Sewerage collection system provided <i>24.7.5.5</i>	
		2. Approval letter from Department of Health and Hospitals 24.5.4.6.7	
		3. Approval letter from TPCG Pollution Control 24.5.4.6.7	
		4. Easements 24.7.5.1	
	e.	General servitudes 24.7.5.1	
		nchmarks: brass or aluminum disk located in the street near the centerline of the road intersection shown on engineering plan 24.7.6.4	эf
	a.	Location	
	b.	Description	
	c.	Elevation msl	
	10 M	Datum used	
		scellaneous compliance	
	a.	Drawings showing final alignment of streets and sewerage, method of sewerage disposal and/or tie-in with existing collective systems, lagoons, lift	
		stations, force mains, etc. 24.7.6.4	
	b.	Sidewalks 24.7.6.5	
		1. Within street right-of-way	
		2. Parallel to the street	
		3. Placement	
		a. Abut the curb -5 ' in width	
		b. Separated from curb – 4' in width	
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Y N N/A	Commercial and/or Light Industrial
	4. Thickness
	a. 4" thick typical
	b. 6" thick at points of vehicle crossings with welded wire fabric
	5. PCC concrete with compressive strength of 4000 psi

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Recommended Runoff Coefficients For Subdivisions

Description of Area	Runoff Coefficients
Business Downtown Neighborhood	0.80 0.50
Residential Single-family Multi-units, detached Multi-units, attached	0.50 0.50 0.65
Residential (suburban)	0.50
Apartment	0.60
Industrial Light Heavy	0.65 0.75
Parks, cemeteries	0.40
Playgrounds	0.25
Railroad yard	0.30
Unimproved	0.20

EXHIBIT NO. 1

Determine the Design Discharge

TRIBUTARY AREA IN ACRES	UNIMPROVED	OPEN SPACE FOR PUBLIC AND INDUSTRIAL USE	RESIDENTIAL	INDUSTRIAL	COMMERCIAL AREAS
UP TO 150	10	10	10	25	25
150 TO 3,000	25	25	25	50	50
OVER 3,000	100	100	100	100	100

EXHIBIT NO. 2

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Use TPR 40 and HDR 35 published by the U.S.N.O.A.A.

MAJOR DRAINAGE ARTERIES

TERREBONNE PARISH, LOUISIANA

Bayou Black

Bayou Blue

Bayou Cane

Bayou Chauvin

Bayou Dularge

Bayou Grand Caillou

Bayou LaCache

Bayou Petit Caillou

Bayou Point Au Chien

CCC Ditch

Chacahoula Bayou

Company Canal

Donner Canal

Falgout Canal

Gulf Intracoastal Waterway

Hanson Canal

Little Bayou Black

Marmande Canal

Minors Canal

Ouiski Bayou

Ringo-Cocke Canal

Six Foot Ditch

St. Louis Bayou

St. Louis Canal

Terrebonne-Lafourche Drainage Canal

Also include any forced drainage pumping station feeder channel.

EXHIBIT NO. 14 8/1/2008

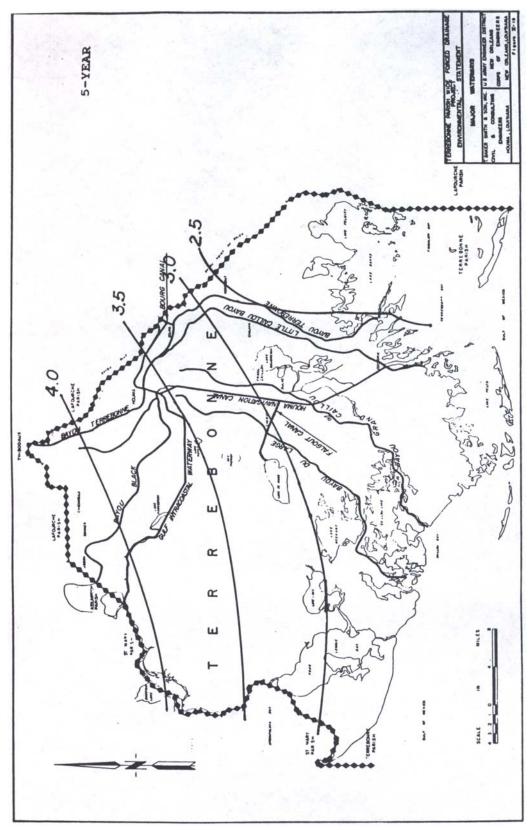
FLOOD ELEVATIONS RESULTING FROM EXTRA-TROPICAL DESIGN STORM

	LEVEE MIN	100YR	25 YR MAX	10 YR		2 YR MAX
PROJECT NAME	EL	MAX EL	EL	MAX EL	EL	EL
1-1A (Bonanza)	4.30	4.21	3.31	2.47	1.76	0.15
1-2 (Ashland)	6.00	3.84	3.59	3.29	3.14	2.74
1-3 (Industrial Blvd)	4.92	3.47	2.50	1.33	0.33	-4.00
1-5 (Bayou Chauvin)	5.00	4.48	3.62	3.02	2.10	0.00
1-7 (Baroid)	6.00	6.45	6.20	5.97	5.64	5.13
1-8 (M&L)	5.10	6.80	6.00	5.22	4.69	3.26
2-1A (Schriever)	1.24	2.92	2.05	1.34	1.22	1.15
2-1B (Summerfield)	10.00	2.59	2.19	1.66	1.33	0.65
3-1B (Boudreaux)	3.00	1.19	1.00	1.00	0.85	0.67
3-1C (Boudreaux)	3.70	2.12	1.67	1.31	1.15	1.02
4-1 (Pnt Aux Chien)	4.00	1.58	1.24	1.02	0.95	0.00
4-2A (Smithridge)	5.00	4.47	4.09	3.80	3.50	3.02
4-7 (Bourg)	4.20	4.73	3.95	3.34	2.85	1.60
4-MONTE (Montegut)	5.00	2.23	1.71	1.26	1.08	1.01
5-1A (Chauvin)	2.50	1.68	1.33	1.08	1.00	0.92
5-1B (Chauvin)	1.10	1.19	1.00	0.91	0.75	0.50
6-1 (Gibson)	4.30	1.16	1.01	0.88	0.74	0.51
6-2A (Donner)	4.20	4.20	4.20	4.20	3.53	0.00
8-2 (Bayou Dularge)	2.80	2.52	1.65	1.16	1.01	1.00
D-38 (Concord Rd)	3.67	3.33	2.40	1.00	0.42	-0.80
D-39 (Barataria)	10.00	6.83	6.26	5.73	5.36	1.87
D-40 (Cenac St)	3.00	1.74	1.47	1.27	1.18	1.04
D-41 (Williams St)	5.00	4.98	4.21	3.49	-1.20	-3.00
HOUMA LAKE S.A.	-	2.03	1.60	1.20	1.04	0.73
OUISKI BAYOU S.A.	-	0.94	0.74	0.60	0.51	0.38
TIGER BAYOU S.A.	-	1.40	0.81	0.65	0.60	0.41
COTEAU-ST LOUIS S.A.	-	2.34	1.82	1.42	1.20	0.82
BULL RUN S.A.	-	1.44	1.12	0.90	0.70	0.50

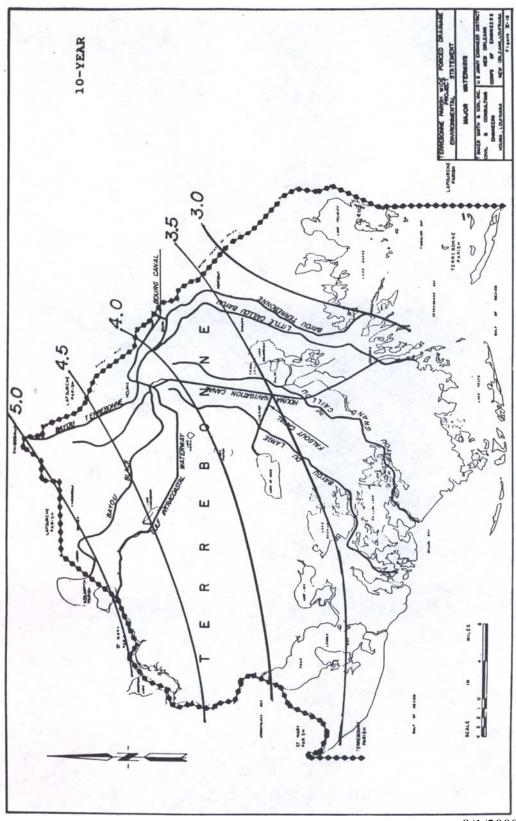
TABLE 4-3. Extra-tropical storm peak pump station reservoir flood elevations.

Check with Engineering Division to see if these elevations have changed.

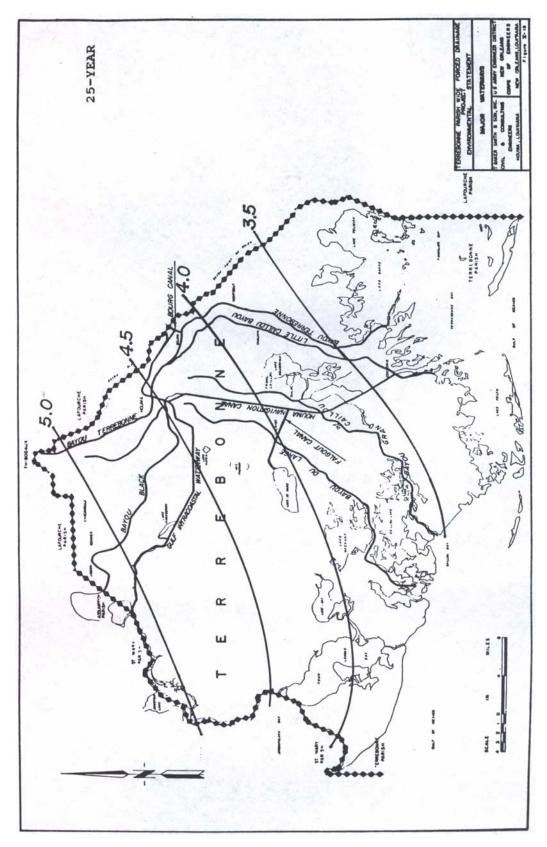
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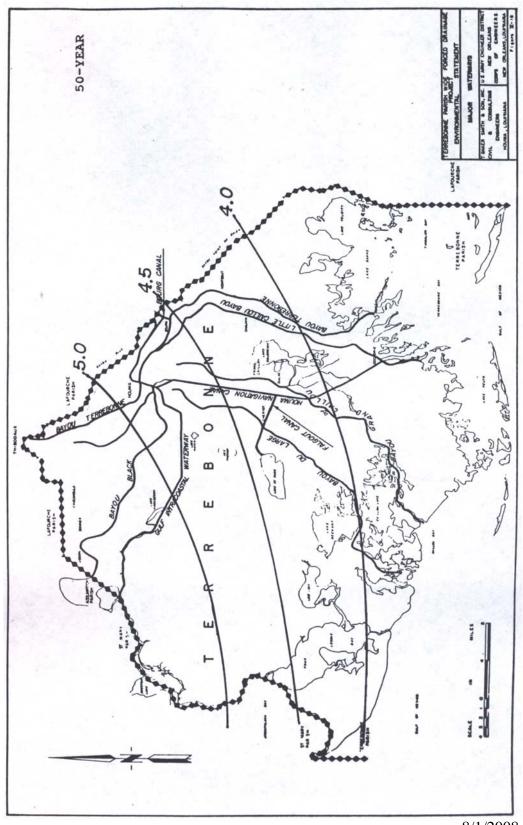
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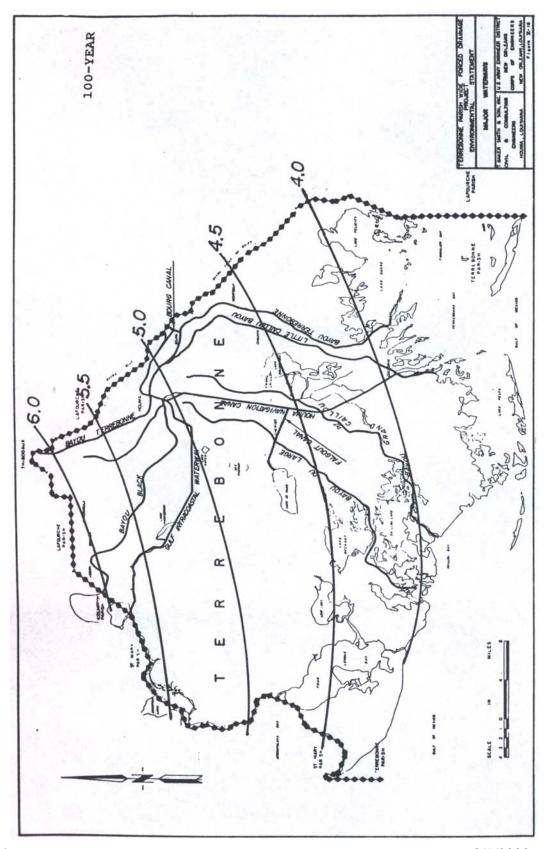
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